

# Garden Bridge - Design

Paul Sanders

1

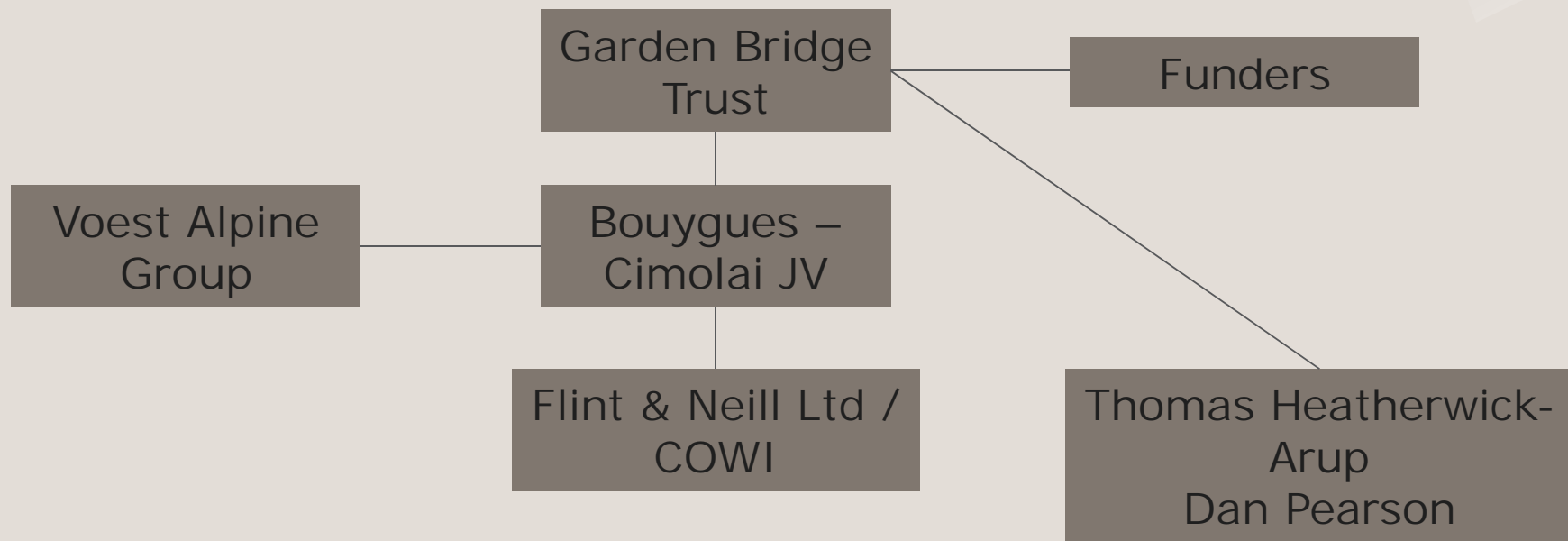
9 JUNE 2016  
GARDEN BRIDGE

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a GOWI company

# Contents

- › Background and inspiration
- › Design
- › Construction
- › Conclusions

## Project Organisation





## Project Vision

- › Garden on the Thames
- › New destination
- › Free for all
- › Horticultural excellence
- › Views of the river and city
- › A new perspective on the city

› Joanna Lumley

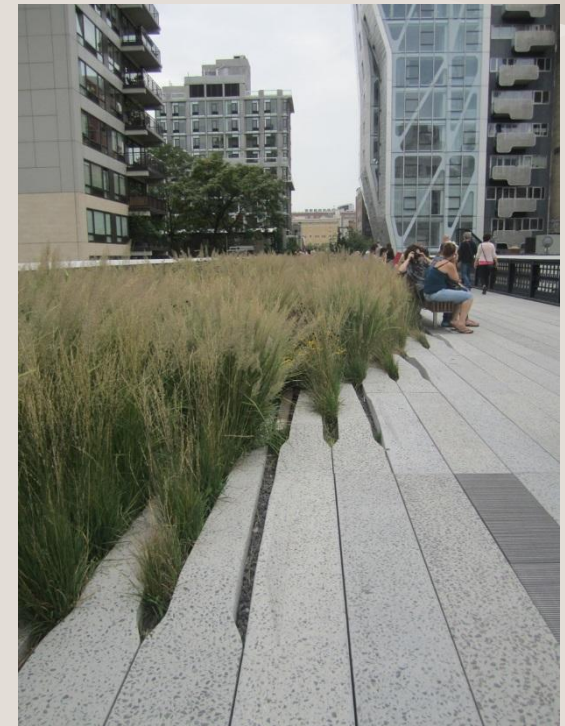


› Thomas  
Heatherwick





# New York High Line Bridge



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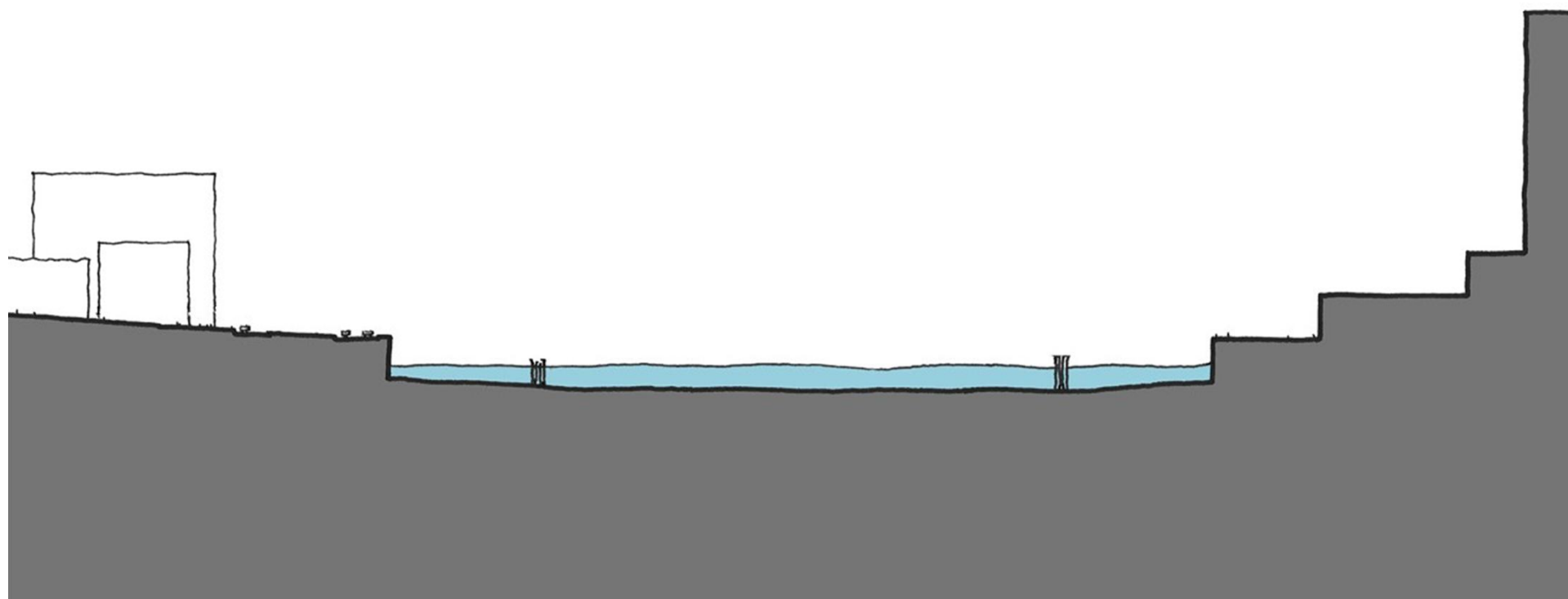


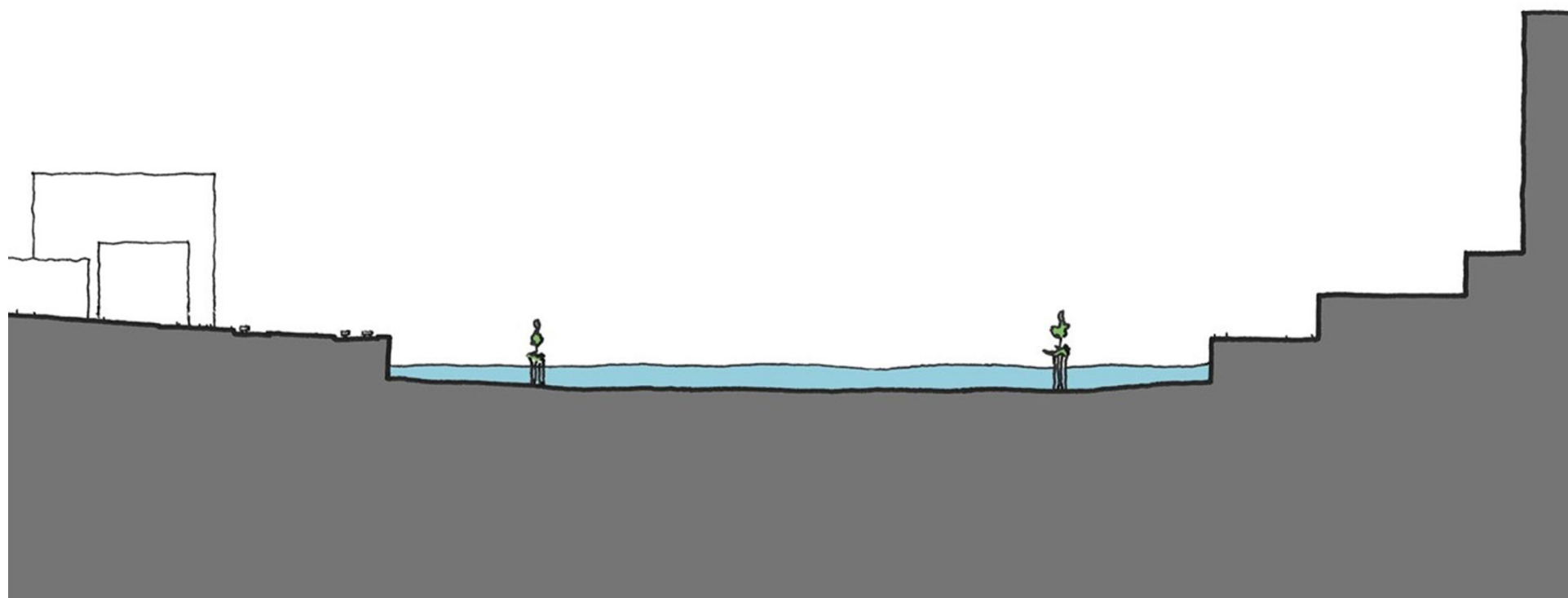




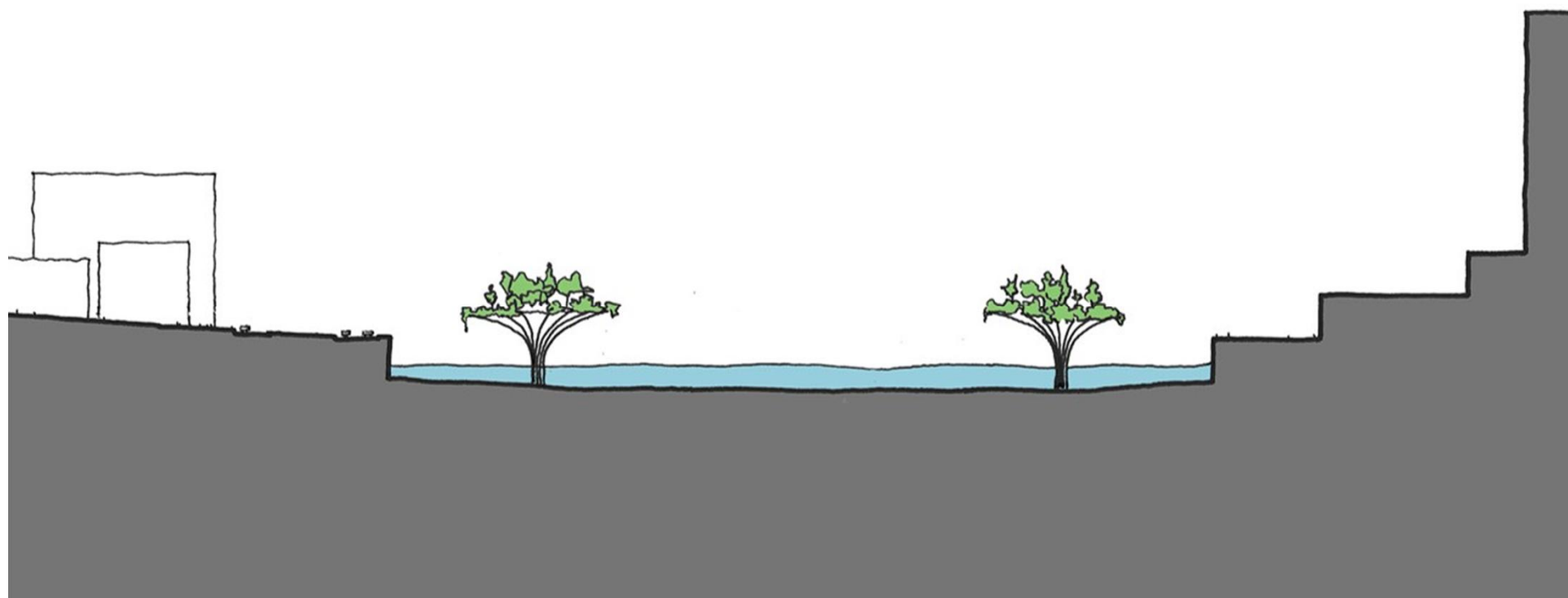
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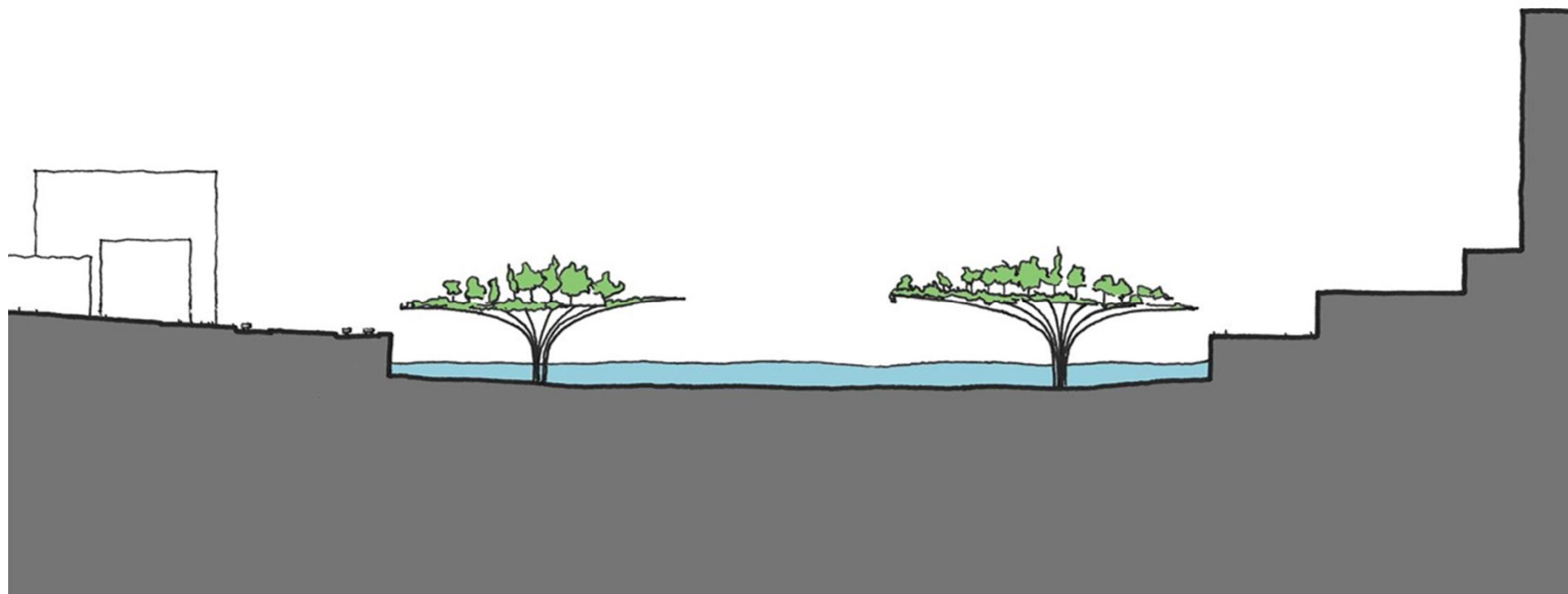
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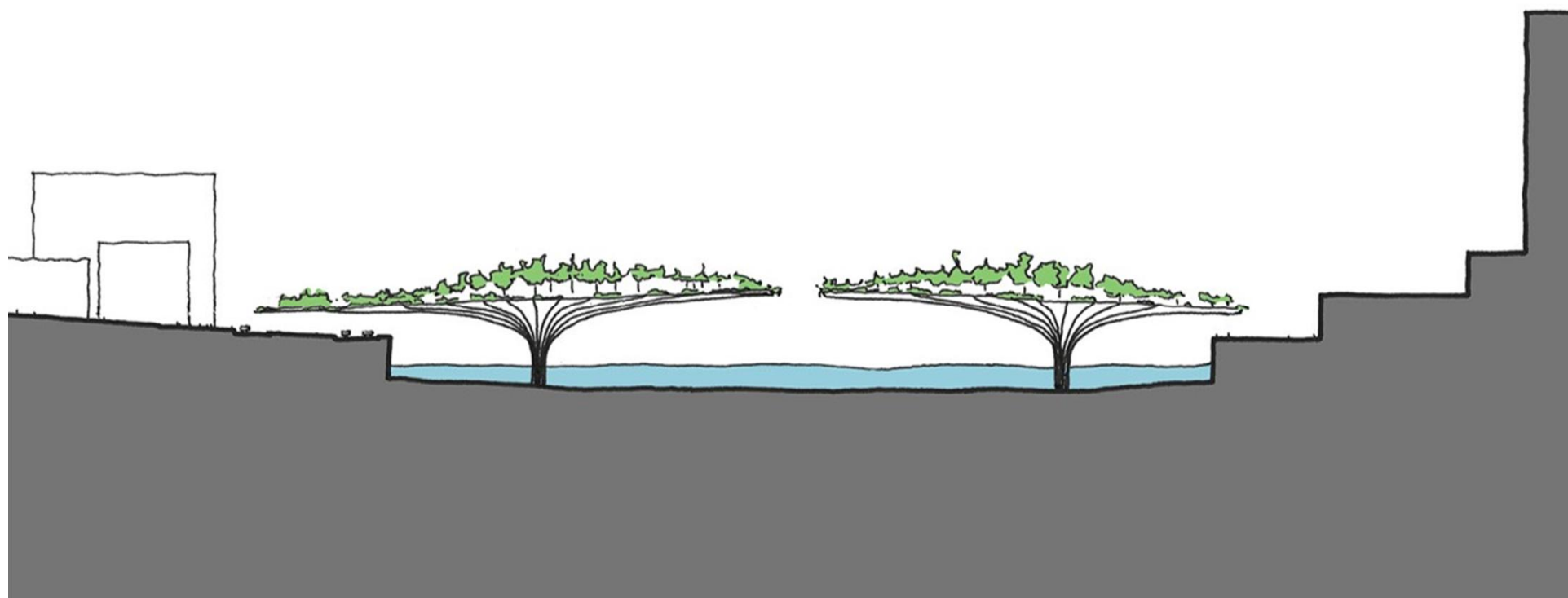








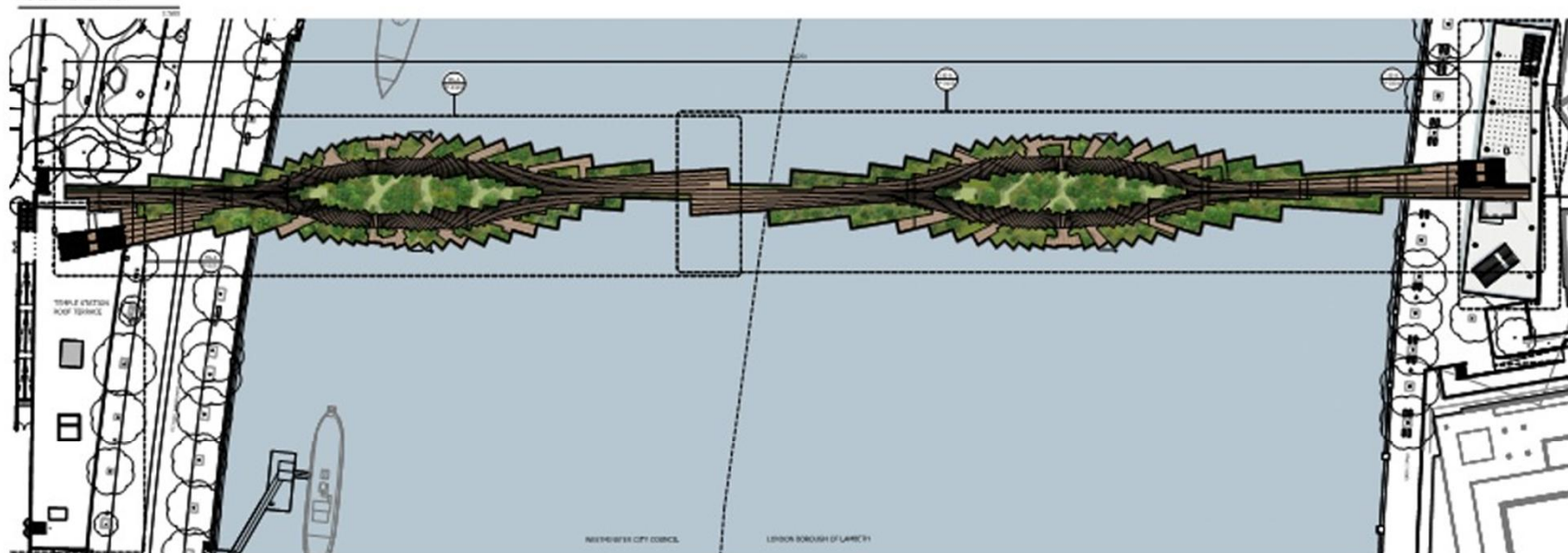




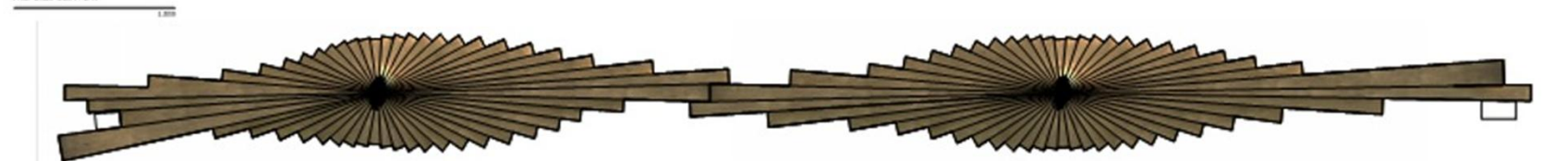




PROPOSED WEST ELEVATION



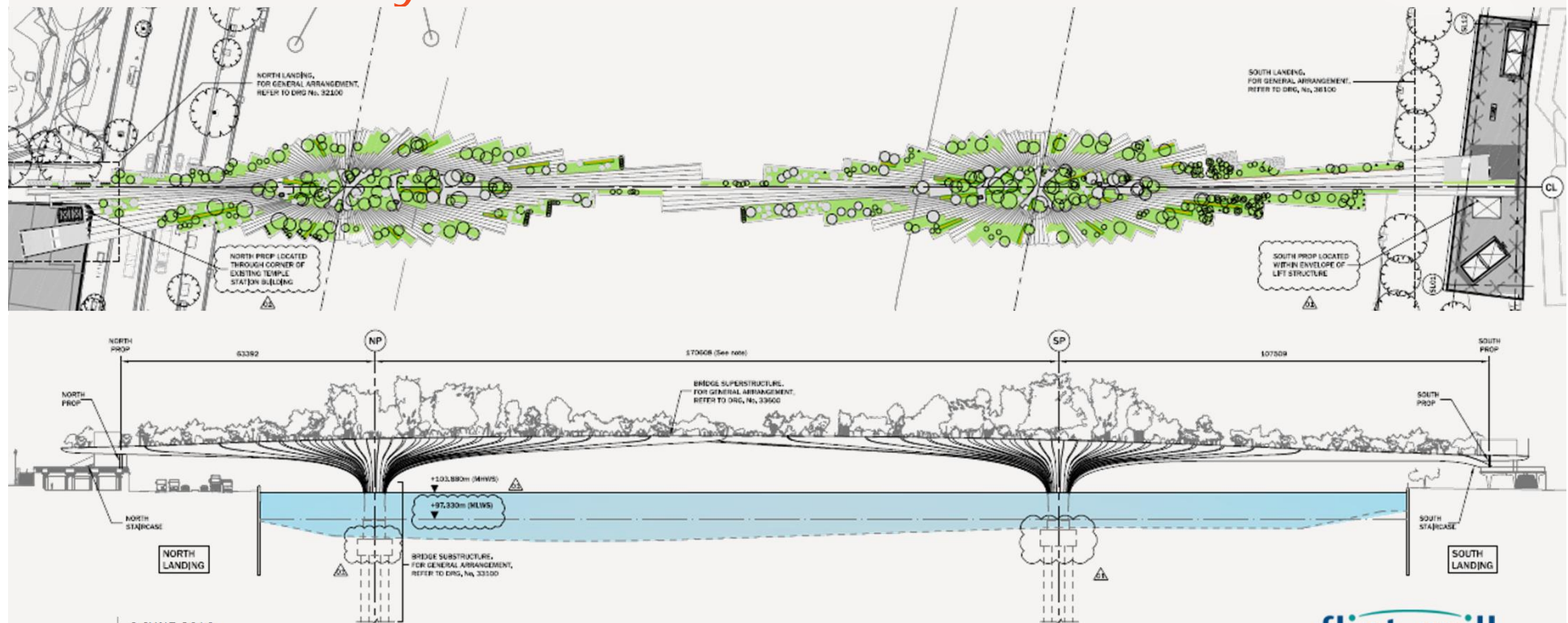
PROPOSED DECK PLAN



PROPOSED REFLECTED SOFFIT PLAN



# Structural System



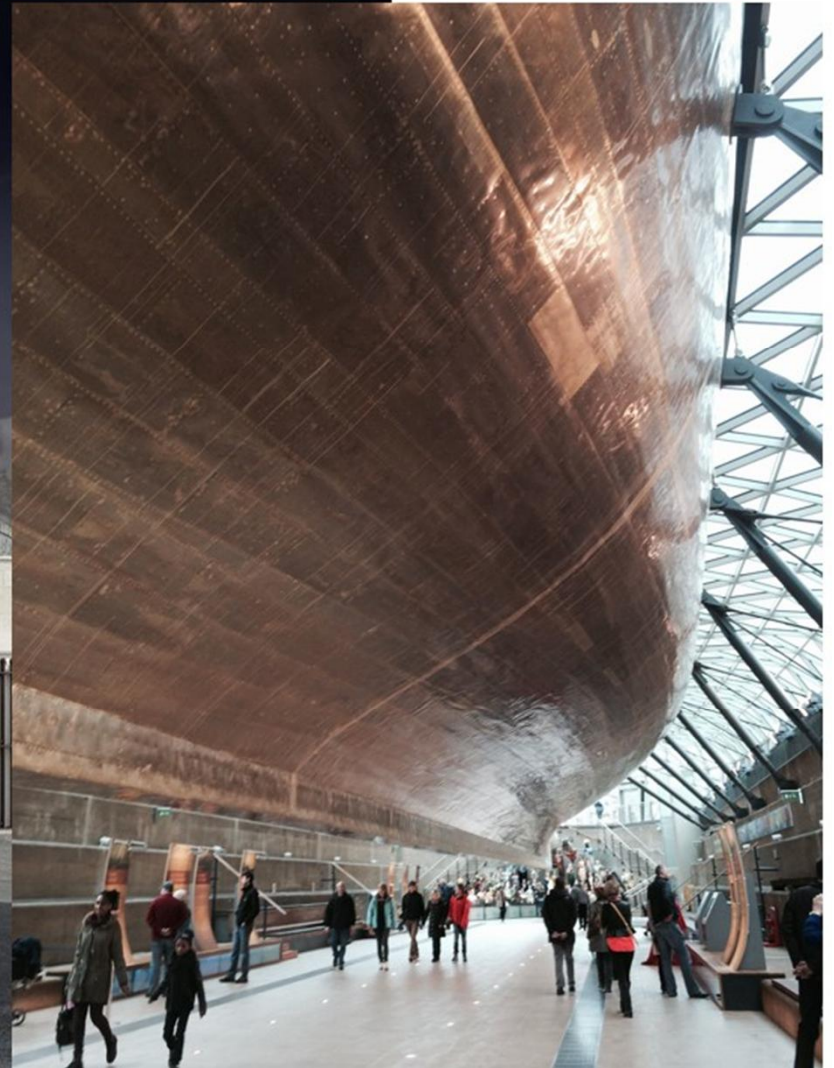
**Table 3:** Estimated annual visitors for similar bridges/attractions

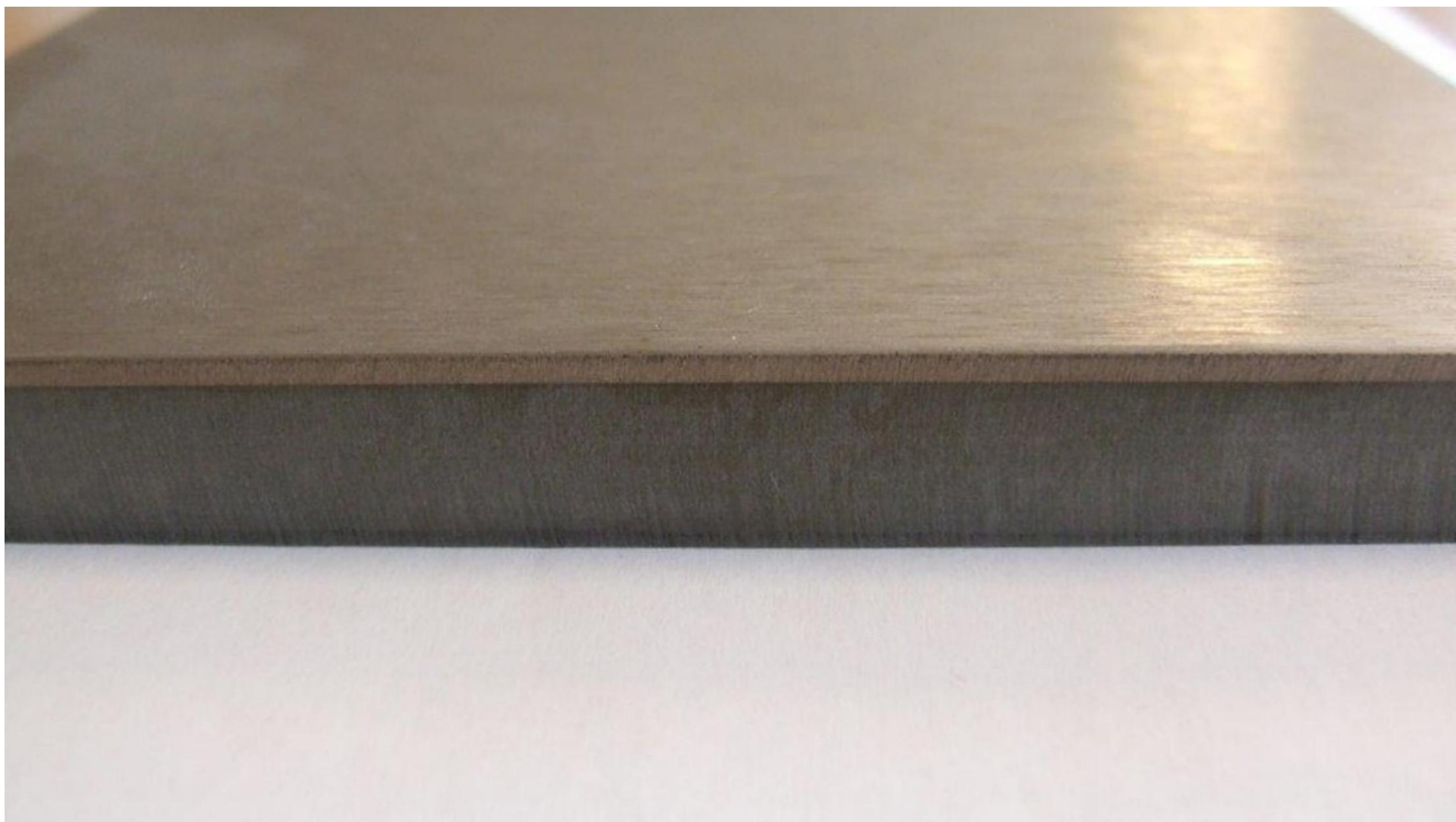
Comparators	Estimated Annual Visitors	Peak Weekday	Peak Saturday	Peak Sunday
Blackfriars Bridge	4.1 million	15,000	8,000	7,000
High Line, New York	4.4 million	3,000-15,000	18,000-20,000	-
Waterloo Bridge	4.9 million	17,000	14,000	12,000
Millennium Bridge	6.1 million	24,000	29,000	17,000
Tower Bridge	7.4 million	27,000	32,000	30,000
Hungerford Bridge (northside + southside)	8.5 million	34,000	49,000	28,000
<b>Garden Bridge<sup>9</sup></b>	<b>7.1 million</b>	<b>27,000</b>	<b>30,000</b>	<b>18,000</b>





Cut Sark, 1869









## Cu-Ni

- › Appearance
- › Durability
- › Resistance to fouling
- › Welding consumables – 70:30 Cu-Ni – tolerate iron dilution

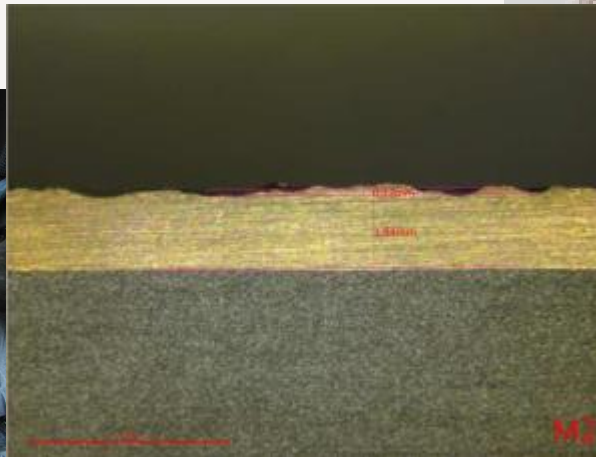


## Trials and Mock-Ups





## Glass Bead Shot Peened

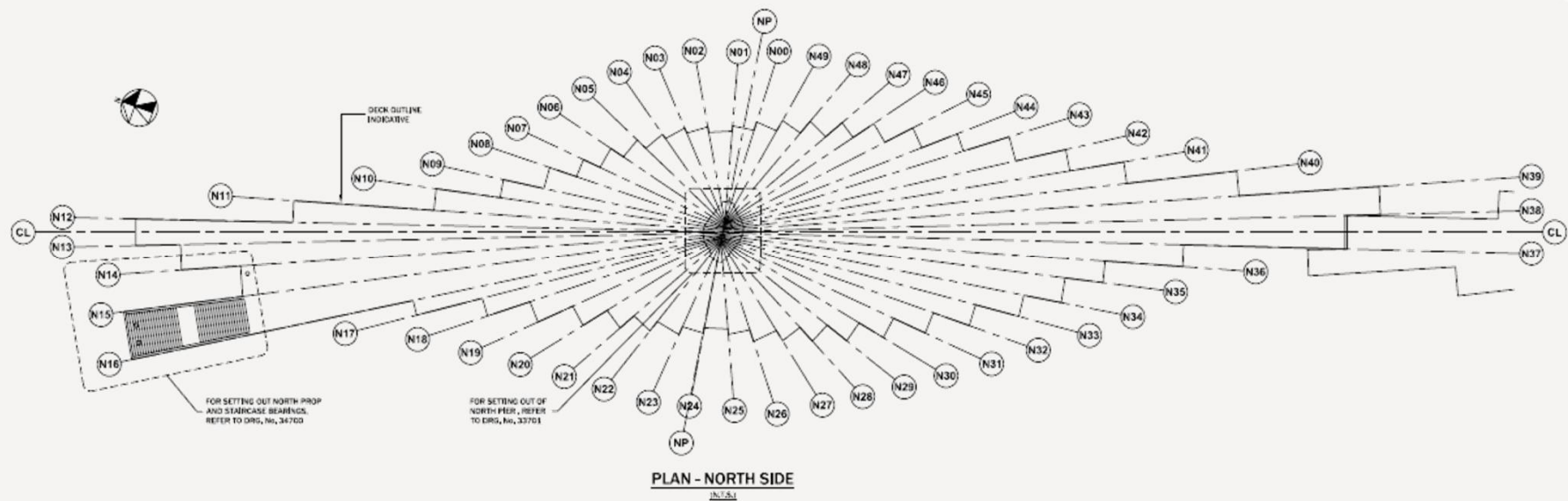


- > Area "M" – Shot peened with 2.85-3.45mm dia.
- > Area "L" - Shot peened with 2.00-2.40 mm dia.

# Plate

- › 90-10 Cu-Ni Rolled bonded
- › 2mm Cu-Ni everywhere except piers which have 5mm Cu-Ni
- › Cu-Ni -  $R=300\text{MPa}$ ,  $R_{p(0.2)} > 100\text{MPa}$
- › Cu-Ni - Elongation at failure =30%
- › Carbon steel plate 355-500MPa

# Structural System



# Garden Bridge Geometry

Soil Deck

Internal  
Truss

Soffit

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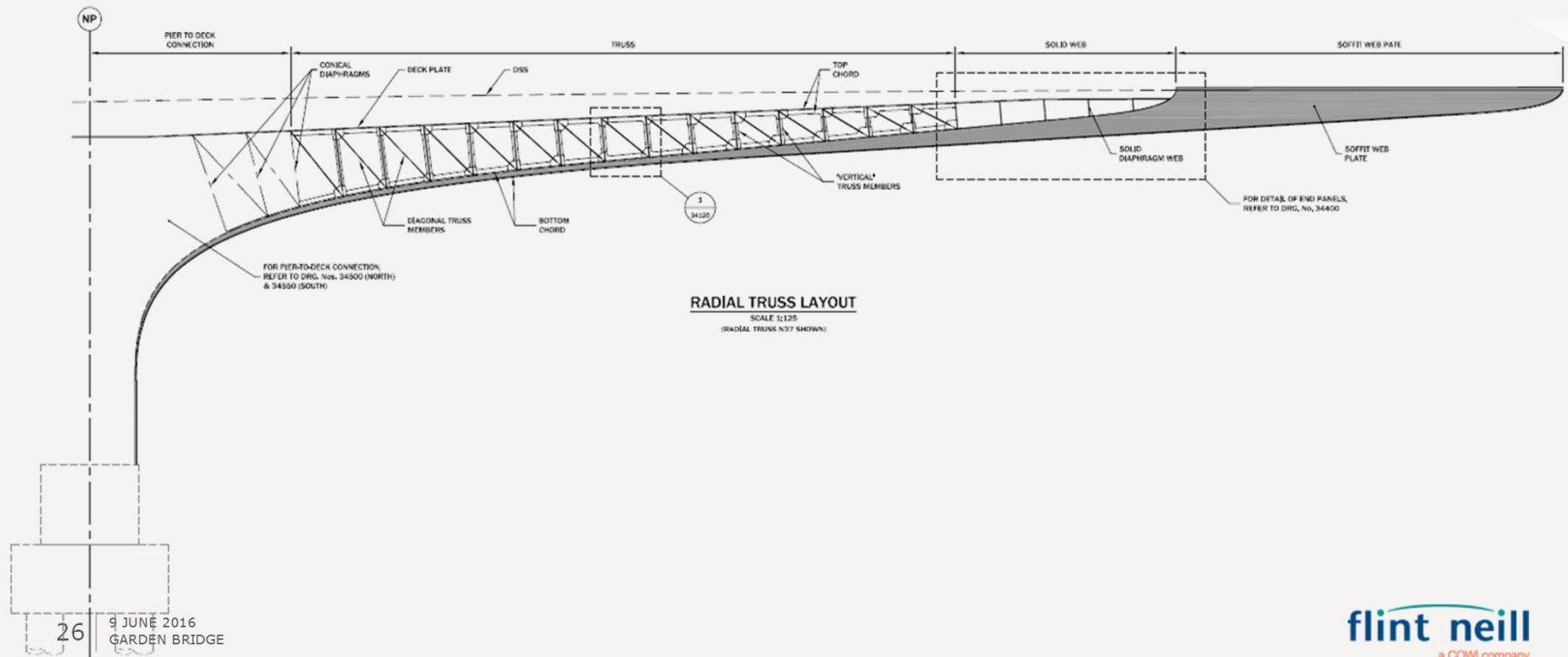
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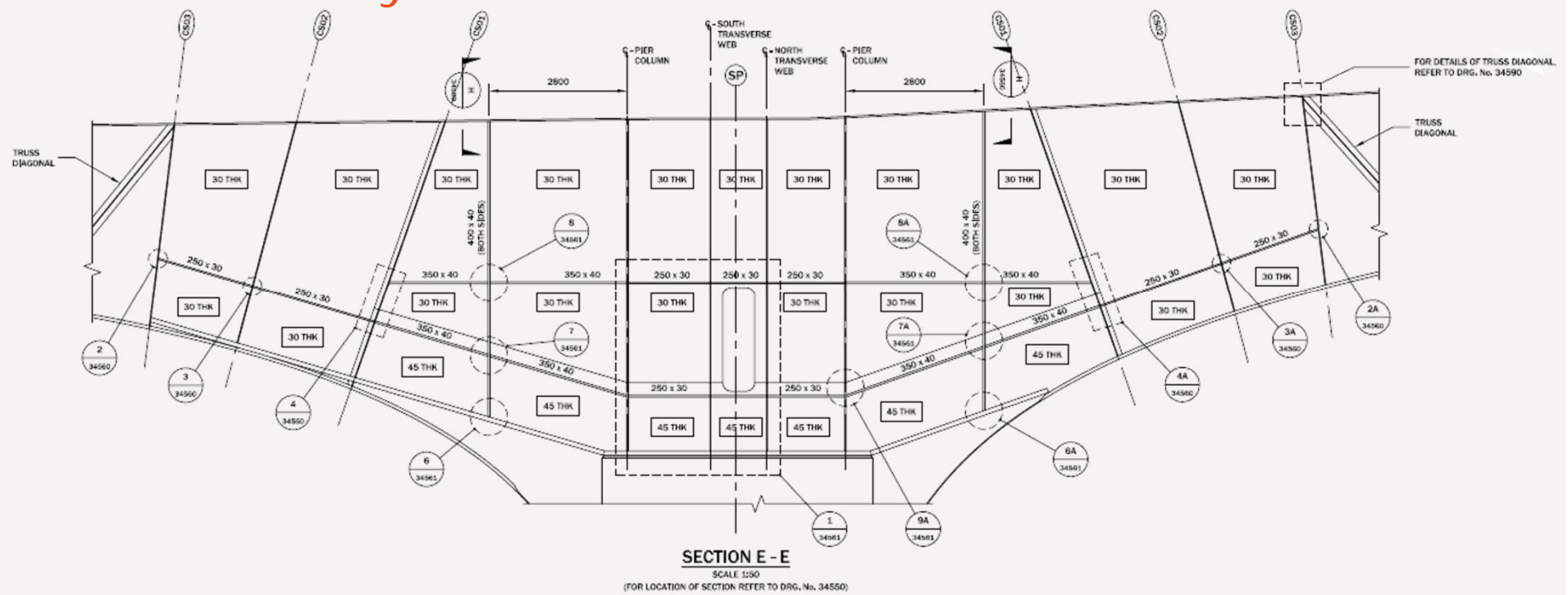


# Structural System



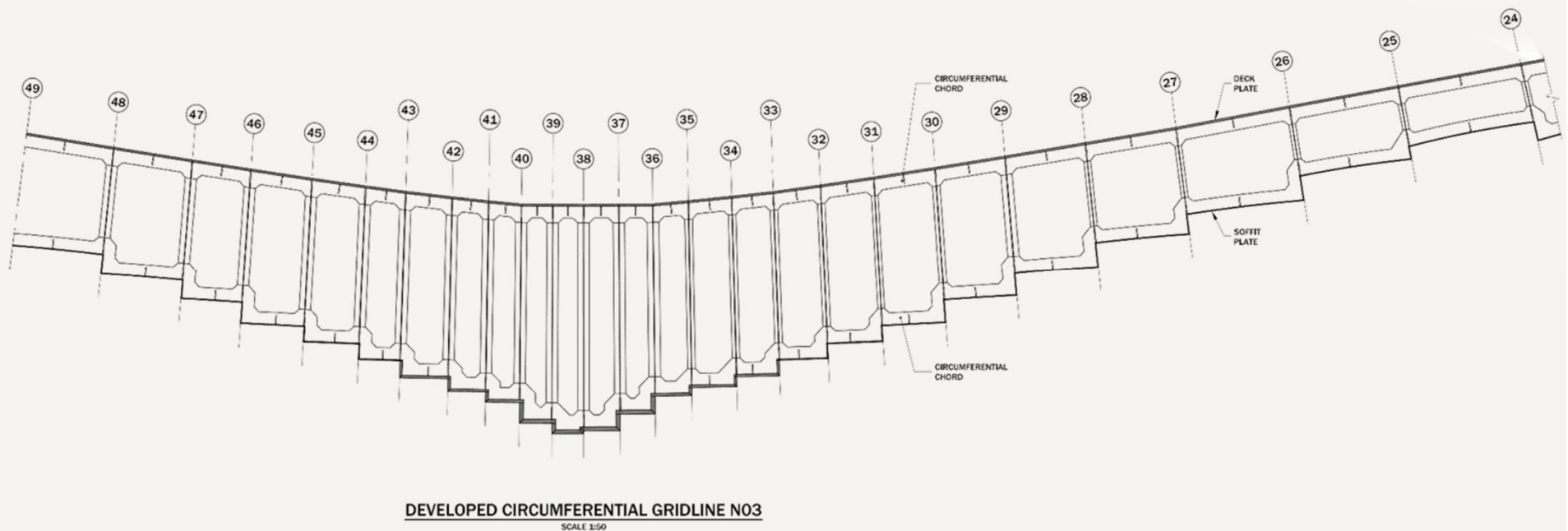


# Structural System

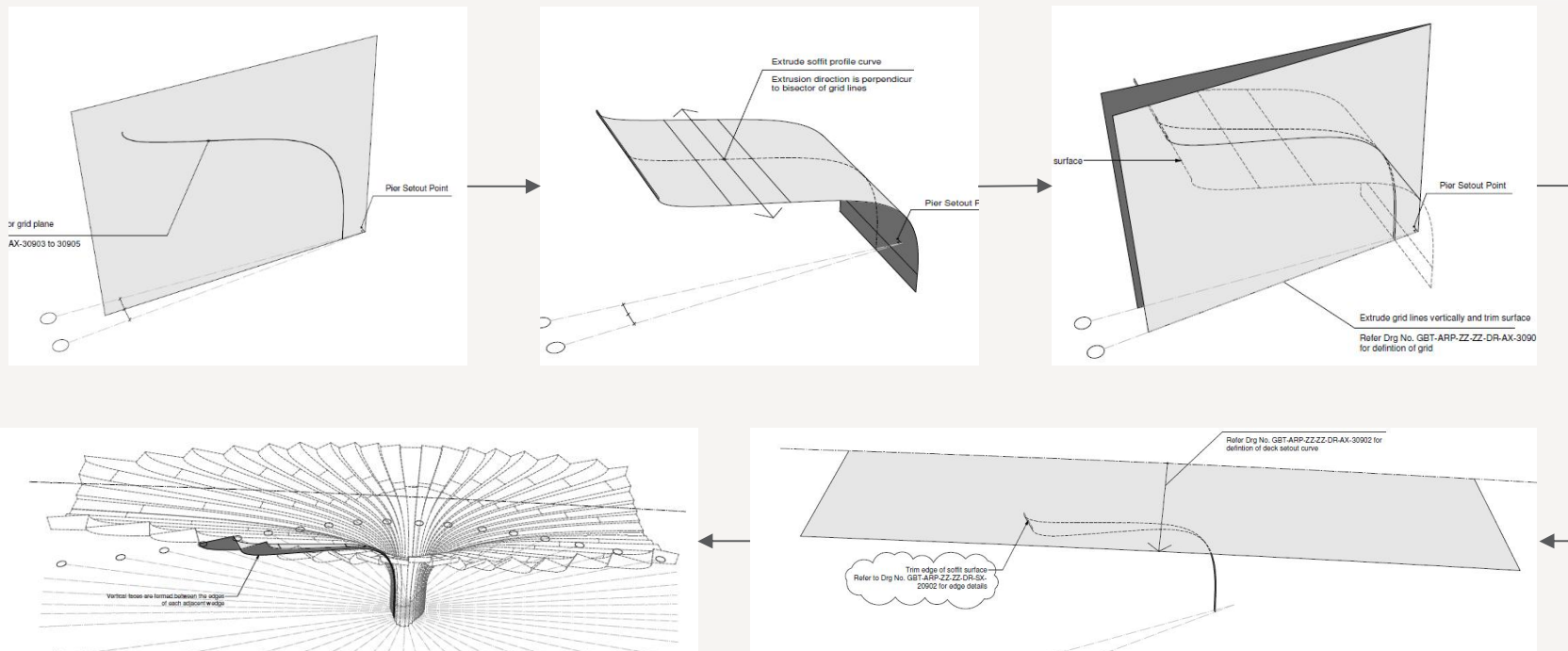




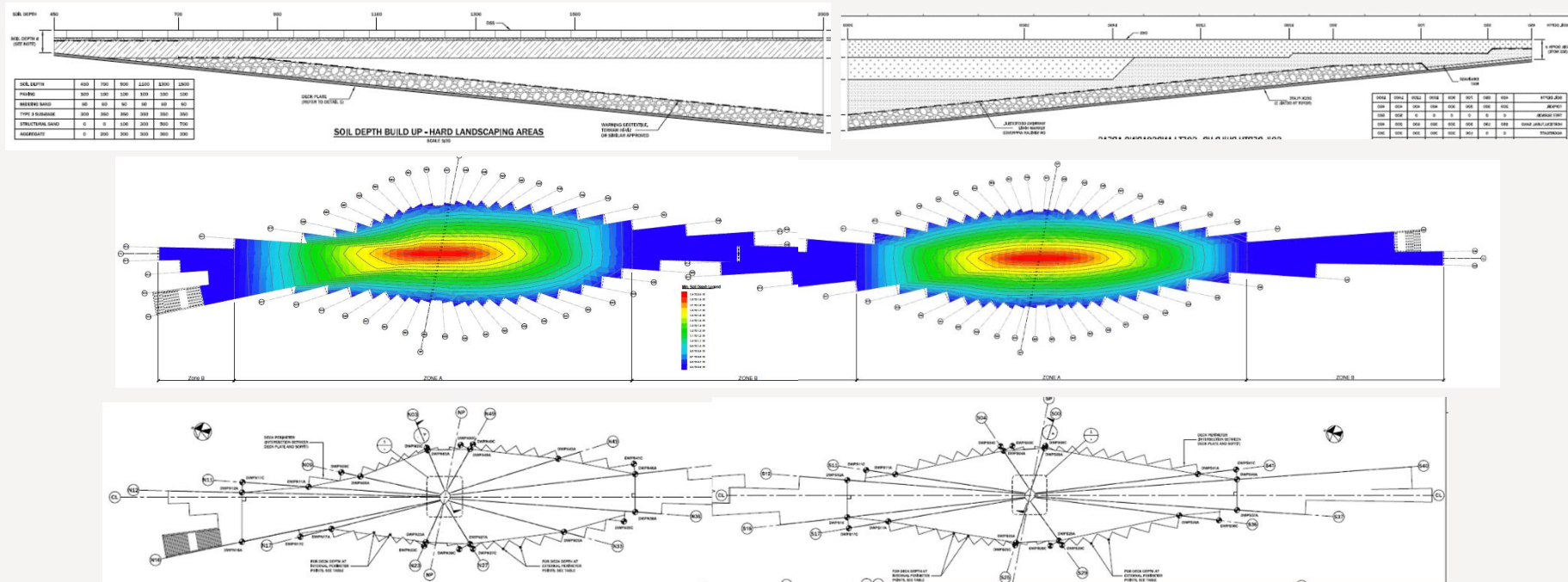
# Structural System



# Garden Bridge Geometry - Soffit

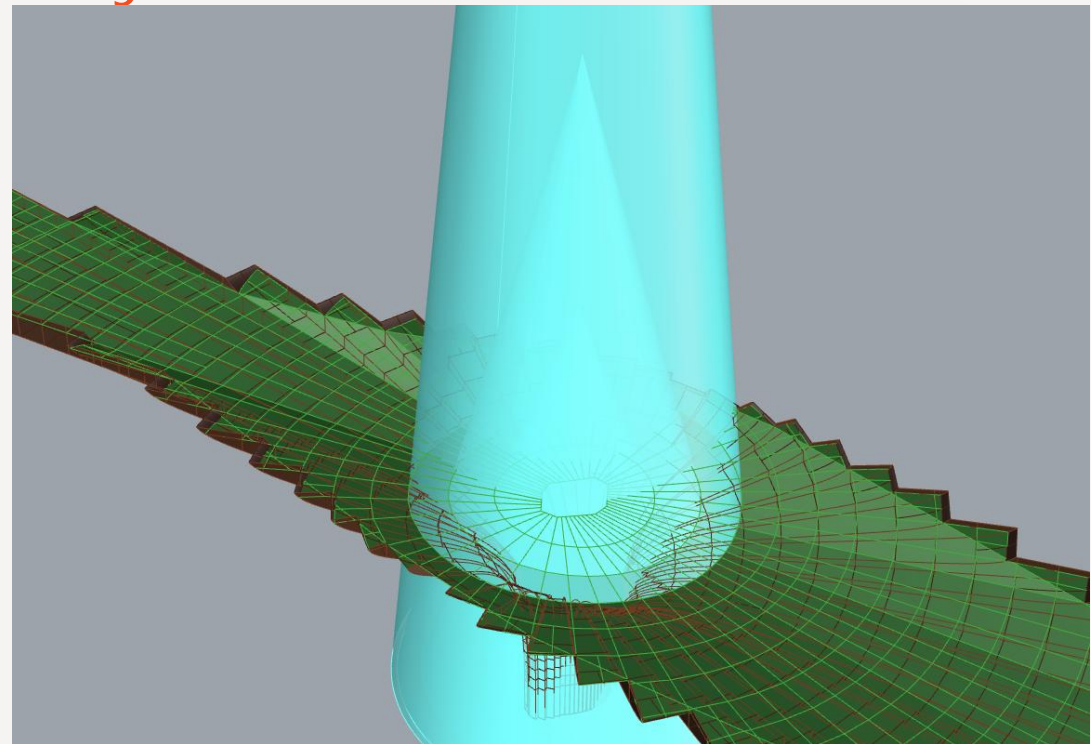
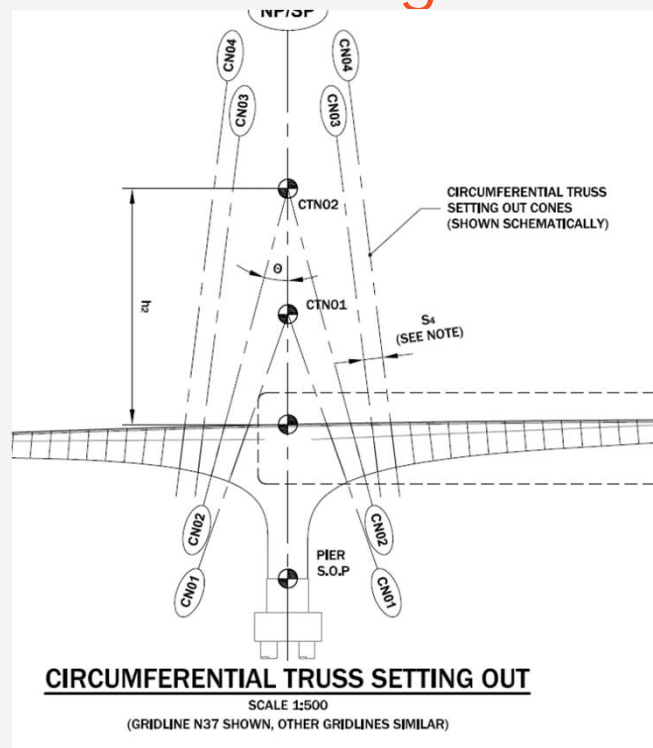


# Garden Bridge Geometry – Soil Deck





## Garden Bridge Geometry – Cones



## Garden Bridge Geometry – Internal Arrangement

Radial  
Chords

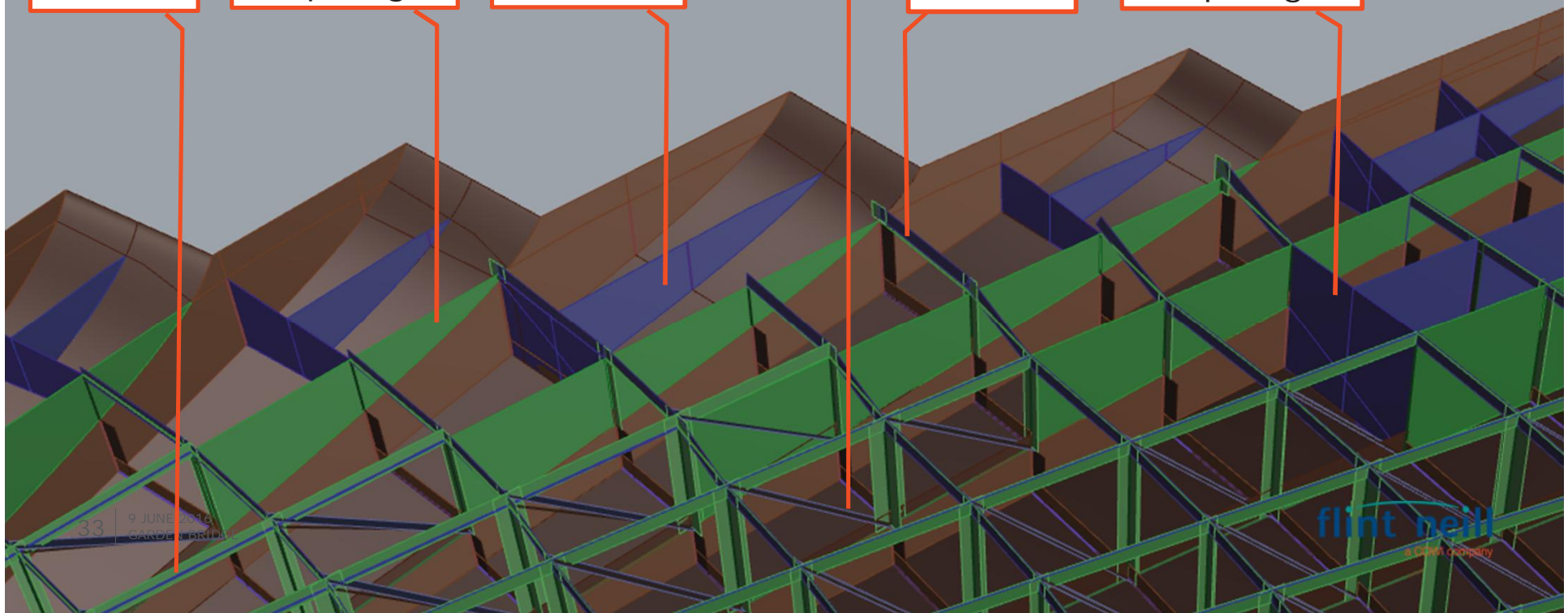
Radial  
Diaphragm

Radial  
Bulkhead

Truss

Circ.  
Chords

Circ.  
Diaphragm



## Garden Bridge Geometry – Internal Arrangement

Octagon

Radial  
Webs

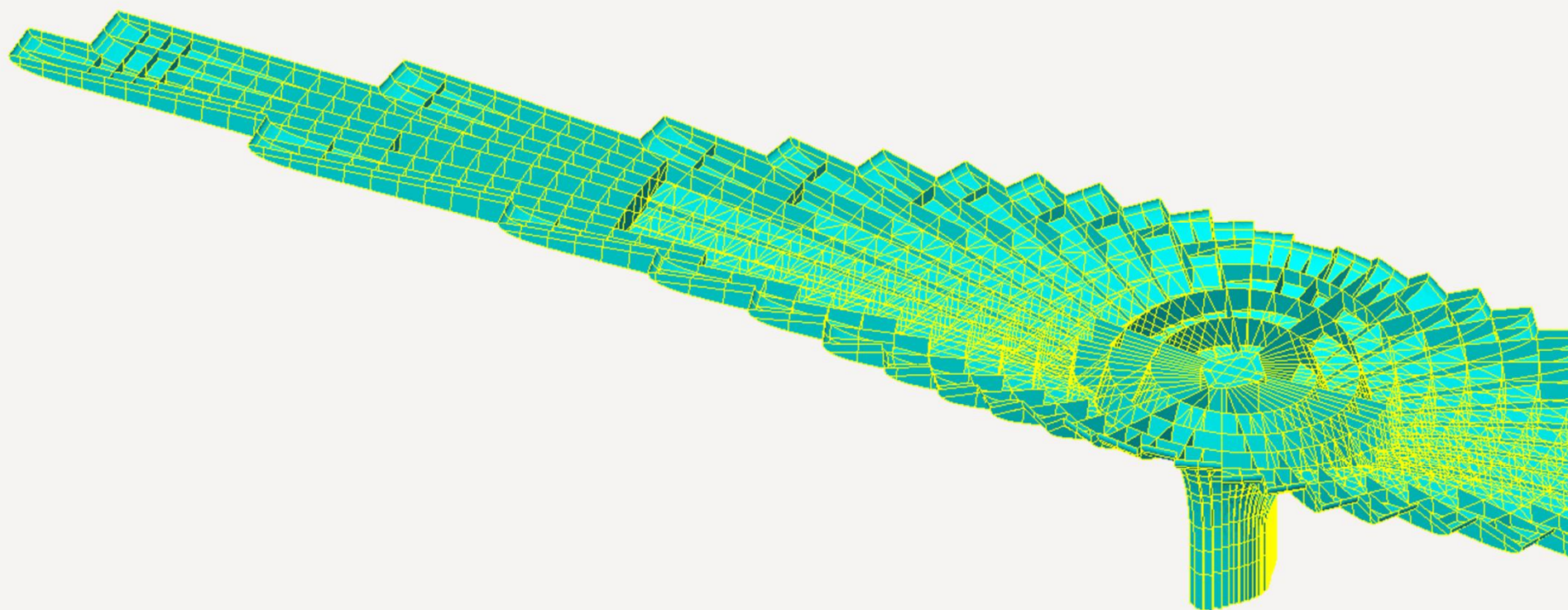
Ring  
Diaphragms

Skirt Plate

Shear Plates



# Analysis



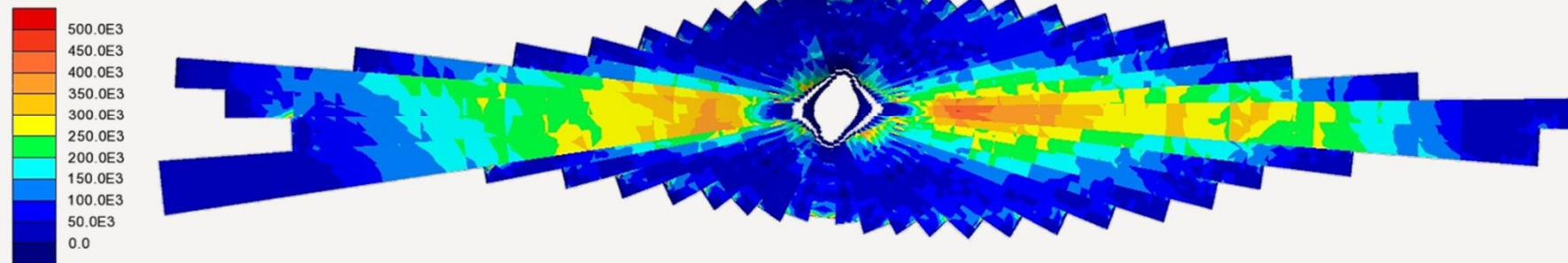


## Results – Soffit Stress

ULS B | DNPSMy+ | Adv UB, closed | 1.1a: DL + LL + Cooling + Snow + (Current)

Entity: Stress (middle) - Thin Shell

Component: SE (Units: kN/m<sup>2</sup>)

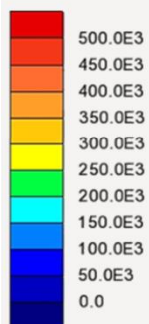


Maximum 1.73055E6 at node 29376 of element 17412

Minimum 24.2713 at node 3497 of element 1002

# Results – Soffit Stress

Analysis: 002 Linear  
Loadcase: 21: Test Loadcase (Copy 1)  
Results file: S009\_007 Nonlinear D-002 Linear.mys  
Entity: Stress (middle) - Thin Shell  
Transformation: Feature  
Component: SE (Units: kN/m<sup>2</sup>)



Maximum 3.36792E6 at node 3180 of element 1159

Minimum 184.026 at node 1745 of element 502

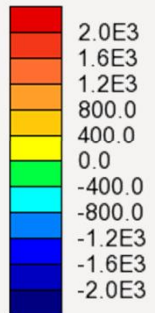
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## Results – Truss Forces

Analysis: 001 Test Analysis  
Loadcase: 1: Test Loadcase  
Results file: S009\_016~001 Test Analysis.mys  
Entity: Force/Moment - Thick 3D Beam  
Component: Fx (Units: kN)



Maximum 2.91017E3 at node 1 of element 8118  
Minimum -116.027E3 at Gauss point 11 of element 8



## Automation of Checking

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Client / Project

Garden Bridge

SUBJECT

Von Mises (Surface) Plot of Utilisation for Soffit Flanges - SOUTH

Job No.

1499

Sub Ref

No.

Date

30/03/16

Prep

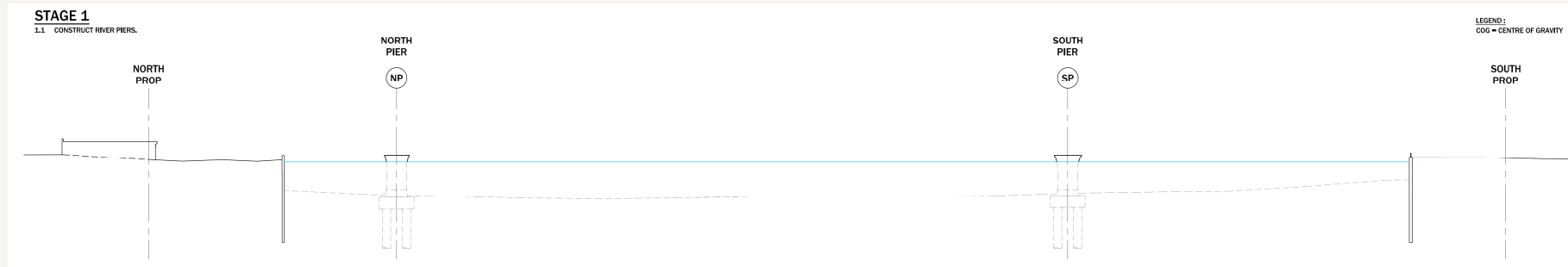
RIJK

Check

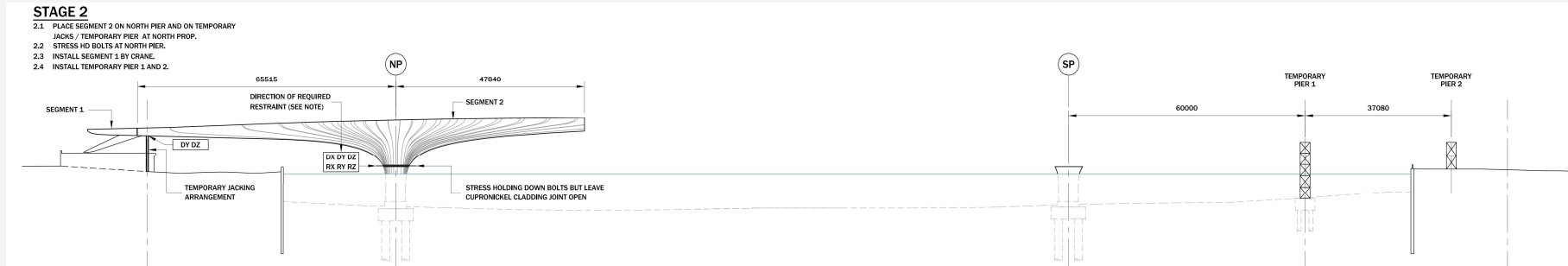
BNCY



## Erection sequence Stage 1

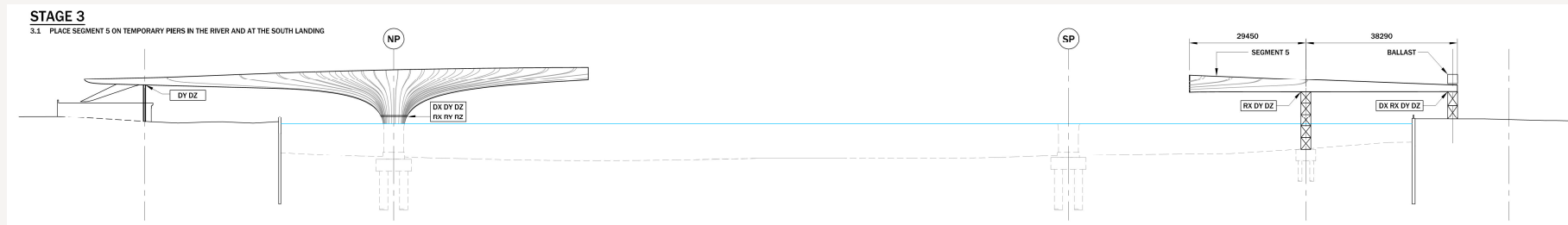


## Erection sequence Stage 2



1. NORTH PROP IS SET LONGER THAN ITS FINAL REFERENCE STRESSED LENGTH TO ALLOW LATER INCREMENTAL JACKING OF THE NORTH PROP. AT STAGE 2 THE FORCE IN THE NORTH PROP IS ZERO UNDER UNFACTORED PERMANENT LOAD.
2. REQUIREMENT FOR TEMPORARY PIER AT NORTH PROP TO BE CONFIRMED BY CONTRACTOR.
3. WEIGHT, COG AND AS-BUILT FABRICATION GEOMETRY OF COMPLETED SEGMENT 1 AND 2 TO BE DETERMINED BY WEIGHING PRIOR TO DELIVERY TO SITE.
4. GEOMETRY OF BRIDGE CANTILEVERS TO BE SURVEYED AFTER STRESSING OF MACALLOY BARS. REACTION FORCE IN NORTH PROP TO BE RECORDED.

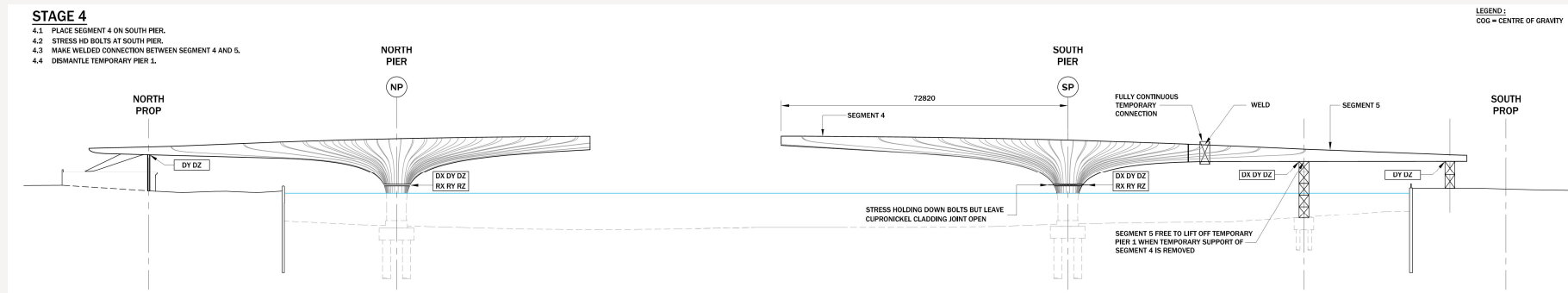
## Erection sequence Stage 3



1. HEIGHT OF TEMPORARY PIERS IN THE RIVER AND THE SOUTH LANDING TO BE ADJUSTABLE VIA TEMPORARY JACKS/PACKERS.
2. BALLAST REQUIREMENT AT TEMPORARY PIER TO BE CONFIRMED.
3. WEIGHT AND COG AND AS-BUILT FABRICATION GEOMETRY OF COMPLETED SEGMENT 5 TO BE DETERMINED BY WEIGHING PRIOR TO DELIVERY TO SITE.



## Erection sequence Stage 4

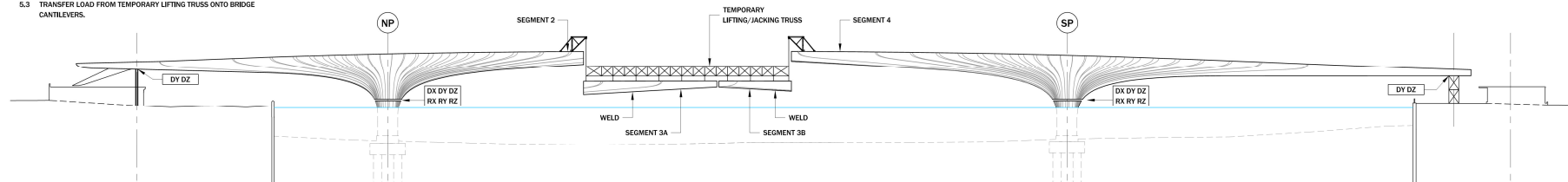


1. WEIGHT AND COG AND AS-BUILT FABRICATION GEOMETRY OF COMPLETED SEGMENT 4 TO BE DETERMINED BY WEIGHING PRIOR TO DELIVERY TO SITE.
2. IN ORDER TO LIMIT SOUTH PIER BENDING MOMENTS, THE SELF-WEIGHT OF SEGMENT 4 MUST REMAIN SUPPORTED BY THE TRANSPORT BARGE UNTIL THE TEMPORARY CONNECTION BETWEEN SEGMENT 4 AND 5 IS COMPLETE.
3. GEOMETRY OF BRIDGE CANTILEVERS TO BE SURVEYED. REACTION FORCE NORTH PROP AND TEMPORARY PIER 2 TO BE RECORDED AFTER DISMANTLING TEMPORARY PIER 1.

# Erection sequence Stage 5

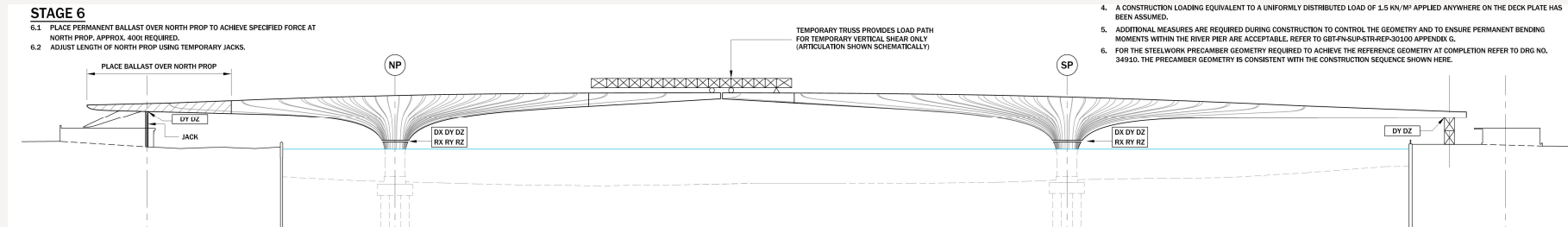
## STAGE 5

- 5.1 STRAND JACK SEGMENT 3A+3B WITH TEMPORARY LIFTING TRUSS.
- 5.2 MAKE WELDED CONNECTION BETWEEN SEGMENT 2 AND 3A, AND BETWEEN 3B AND 4, LEAVING JOINT OPEN BETWEEN 3A AND 3B.
- 5.3 TRANSFER LOAD FROM TEMPORARY LIFTING TRUSS ONTO BRIDGE CANTILEVERS.



1. WEIGHT AND COG AND AS-BUILT FABRICATION GEOMETRY OF COMPLETED SEGMENTS 3A AND 3B TO BE DETERMINED BY WEIGHING PRIOR TO DELIVERY TO SITE.
2. GEOMETRY OF BRIDGE CANTILEVERS TO BE SURVEYED. REACTION FORCE NORTH PROP AND TEMPORARY SOUTH LANDING PIER TO BE RECORDED AFTER UNLOADING TEMPORARY LIFTING TRUSS.

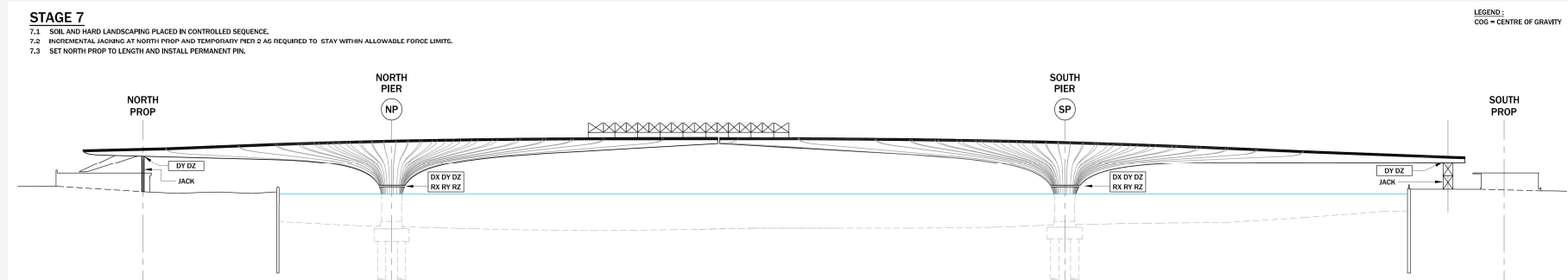
# Erection sequence Stage 6



1. GEOMETRY OF BRIDGE CANTILEVERS TO BE SURVEYED. REACTION FORCE IN NORTH PROP TO BE MONITORED.
2. THE LENGTH OF THE NORTH PROP IS ADJUSTED TO MAINTAIN ZERO FORCE IN THE PROP UNDER UNFACTORED PERMANENT LOAD.



## Erection sequence Stage 7



1. JACKING AT NORTH PROP AND TEMPORARY PIER 2 REQUIRED TO ACHIEVE BALANCED CONDITION WITH ZERO ROTATION OR BENDING MOMENT IN THE PIERS IN THE COMPLETED STRUCTURE.
2. GEOMETRY OF BRIDGE CANTILEVER ENDS TO BE SURVEYED AT FREQUENT INTERVALS DURING SOIL PLACEMENT.
3. MONITOR REACTION FORCES IN NORTH PROP AND TEMPORARY PIER 2.
4. SOIL FILL MATERIAL PROCURED FROM SINGLE SUPPLIER.
5. EACH BATCH OF AS-SUPPLIED SOIL FILL MATERIAL TO BE DENSITY TESTED.
6. VOLUME AND LOCATION OF SOIL PLACEMENT ON EACH CANTILEVER TO BE DOCUMENTED AND SUBJECT TO INSPECTION ON SITE.

## Headline Quantities

- › Steel 5,200 Tonnes
- › Roll bonded plate 1,700 Tonnes – up to 65mm thick
- › 10,000 Tonnes of soil
- › 65m long piles  $\approx 2,070\text{m}^3$
- › Piers and Pile caps  $\approx 1,760\text{m}^3$

# Summary

- › Iconic new bridge for Central London
- › Destination!
- › Durable aesthetically pleasing end structure
- › Novel use of roll bonded plates in exposed civil engineering structure
- › Complex geometry and structural behaviour

Thank you!

